

A Study on Preparing Of High Performance Concrete Using Silica Fume and Fly Ash

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*---***ABSTRACT***--*

The main objective of this study was to investigate the physical properties of High Performance Concrete (HPC) using silica fume and fly ash as mineral admixtures along with the addition of glass fibres. High Performance Concrete (HPC) mixes incorporated various doses of silica fume (0%, 5%, 7.5%, 10% and 12.5%) as partial replacement of cement. Another concrete mixes incorporated various doses of silica fume (0%, 5%, 7.5%, 10% and 12.5%) and 10% fly ash as partial replacement of cement. For all the above mixes a 0.3% of glass fibres is added by volume fraction. The HPC mix, grade M75 with a water-cement ratio of 0.26 is adopted and it is designed as per ACI 211.4R-08.To improve the workability of concrete, super plasticizer of 0.8% by weight of cement is added. The mechanical behaviour were expressed in terms of compressive, split-tensile and flexural strength. The experimental results show that the specimens containing 10% silica fume and 0.3 % glass fibres and another mix containing 10% silica fume, 10% fly ash and 0.3 % glass fibres experienced higher mechanical properties compared to that of the control specimen.

Keywords:- Compression Strength, Fly Ash , Flexural strength, Silica Fume, Split-tensile strength. **---**

I. Introduction

1.1 General

High performance concrete is used for concrete mixture which possess high workability, high strength, high modulus of elasticity, high density, high dimensional stability, low permeability and resistance to chemical attack.

Normal concrete relatively have low strength and elastic modulus are the result of high heterogeneous nature of structure of the material, particularly the porous and weak transition zone, which exists at the cement pasteaggregate interface. By densification and strengthening of the transition zone, many desirable properties can be improved many fold. A substantial reduction of quantity of mixing water is the fundamental step for making HPC. Reduction of w/c ratio will result in high strength concrete. But reduction in w/c ratio to less than 0.3 will greatly improve the qualities of transition zone to give inherent qualities expected in HPC.

To improve the qualities of transition zone, use of silica fume is also found to be necessary. Silica fumes becomes a necessary ingredient for strength above to 80 MPa. The best quality fly ash and GGBS may be used for other nominal benefits. Inspite of the fact that these pozzolanic materials increase the water demand, their benefits will out weigh the disadvantages. The crux of whole problem lies in using very low w/ c ratio, consistant with high workability at the time of placing and compacting. Adopting w/ c ratio in the range of 0.25 to 0.3 and getting a high slump is possible only with the use of superplasticizer. Therefore, use of appropriate superplasticizer is a key material in making HPC.

1.2 Silica Fume

Silica fume is obtained from ELKEM INDIA PVT LTD.

1.2.1 Physical Properties of Silica Fume

The physical properties of Silica Fume (SF) are the particle size, colour, oversize, specific gravity, etc. are explained briefly:-

Particle Size: - SF particles are smooth, spherical size is 1/100 the diameter of Portland cement particle and average particle diameter lies between 0.1 to 0.2 micron range.

Fineness:- the specific surface area of SF as measured by nitrogen absorption method usually lies between 13sq.m/g to 28 sq.m/g. Generally the SF has the fineness value of about 22 sq.m/g.

Colour:- The colour of SF depends on carbon content, lower the carbon content of SF, the lighter is shade of grey. Usually, ferrosilicon furnaces manufacturing low silicon content alloys shoe darker silica fume. Specific Gravity: - The specific gravity of SF produced from high quality silicon and high grade ferrosilicon alloys typically ranges between 2.2 and 2.3.

1.2.2 Chemical Properties of Silica Fume

1.3 Fly ash

Flyash is collected from Mettur Thermal Power Plant (MTTP) Mettur. Specific Gravity is 1.89. The properties of fly ash are shown in table 2. Table 2 Chemical Composition of Fly ash

1.4 Glass Fibres

Fig. 1 Glass Fibres

II. Mix Design

2.1 General

The concrete used in this study was proportioned to attain strength of **75 MPa**. ACI committee recommendation has been used for M75 design The mixes MSG1, MSG2, and MSG3 were obtained by replacing 5, 7.5 and 10 percent of the mass of cement by Silica Fume. Then mix MSFG1, MSFG2 and MSFG3 were obtained by replacing the mass of cement by the above percentage of Silica Fume and with 10% of Fly Ash. The water cement ratio (w/c) is taken as 0.26**.** The mix design has been adopted as per ACI 211.4R-93.

2.2 Material Properties

Characteristic compressive strength = 75 MPa Maximum size of aggregate used $= 12.5$ mm (passing through and retained on 10 mm sieve). Specific gravity of cement $= 3.15$ Specific gravity of fine aggregate $= 2.65$ Specific gravity of coarse aggregate $= 2.77$ Dry rodded bulk density of $FA = 1701.11 \text{ kg/m}^3$ Dry rodded bulk density of $CA = 1692.73$ kg/m³ Slump assumed $=$ 50-75 mm

2.3 Calculation Of Weight Of Coarse Aggregate

From Table 4.3.3 of ACI 211.4R-93, Fractional volume of oven dry rodded $CA = 0.68$ m³ Weight of CA = 1692.73 x $0.68 = 1151.06$ kg/m³

2.4 Calculation Of Quantity Of Water

From Table 4.3.4 of ACI 211.4R-93, For CA of 12.5 mm and slump of 50-75 mm The mixing water $= 148$ ml Void content of FA for this mixing water $= 35$ % Void content of fine aggregate,

$$
V = 1 - \{ \frac{dry\,raded\,of\,FA}{density\,of\,water * Gfa} \}^* \,100
$$

 $V = 35.81 %$

Adjustment in mixing water = $(35.81 - 35)$ x 4.55 = 3.686 ml Total water required = $148 + 3.686 = 151.686$ ml.

2.5 Calculation Of Weight Of Cement

Target mean strength $f_{cr} = 75 + 9.65 = 84.65$ MPa (12277.4 Psi) Water / cement ratio $= 0.26$ Weight of cement $(Kg) = 583.41$ kg/m³

2.6 Calculation Of Fine Aggregate

CEMENT = 583.41 / (3.15 x 1000) = 0.1852 m³ WATER = $151.686 / (1 \times 1000) = 0.152$ m³ $CA = 1151.06 / (2.77 \times 1000) = 0.416 \text{ m}^3$ Entrapped air $= 2\%$ Total volume = $0.1852 + 0.152 + 0.416 + 0.02 = 0.773$ m³ Volume of $FA = 1 - 0.773 = 0.227$ m³ Weight of $FA = 0.227x2.65x1000 = 601.55$ kg/m³

2.7 MIX RATIO

Therefore, the mix ratio adopted is; **1 : 1.03 : 1.973 : 0.26**

Mix	% Silica Fume	% Fly Ash	% Glass Fibre	W/C Ratio
MSG1			0.3	0.26
MSG ₂	7.5		0.3	0.26
MSG3	10		0.3	0.26
MSG4	12.5		0.3	0.26
MSFG1		10	0.3	0.26
MSFG2	7.5	10	0.3	0.26
MSFG3	10	10	0.3	0.26
MSFG4	12.5	10	0.3	0.26
\curvearrowright				0.26

Table 4 Mix Proportions

III. Experimental Investigation

3.1 General

Strength studies on HPC (M75) were conducted by means of compressive, tensile and flexural strength for 28 days respectively.

3.2 Compressive Strength Test

The test is carried out on 150x150x150 mm size cubes, as per IS: 516-1959. The test specimens are marked and removed from the moulds and unless required for test within 24 hrs, immediately submerged in clean fresh water and kept there until taken out just prior to test. A 2000 KN capacity Compression Testing Machine (CTM) is used to conduct the test. The specimen is placed between the steel plates of the CTM and load is applied at the rate of 140 Kg/Cm²/min and the failure load in KN is observed from the load indicator of the CTM. Compressive strength = $($ Load / Area) N/mm²

3.3 Split Tensile Strength Test

The splitting tensile strength of concrete cylinder was determined based on 516-1959. The load shall be applied nominal rate with in the range 1.2 $N/(mm^2/min)$ to 2.4 $N/(mm^2/min)$

Split Tensile Strength $=$ *LD P* π 2

Fig. 3 Graphical Representation of Split Tensile Strength of Cylinder at 28 Days.

3.4 Flexural Strength Test

The flexural strength of concrete prism was determined based on IS: 516 –1959. Place the specimen in the machine in such a manner that the load is applied to the upper most surface as cast in the mould along two lines spaced 13.3cm a part. Apply load without shock and increase continuously at a rate of 180 kg/min and it is increased until the sample fails. Measure the distance between the line of fracture and nearest support. If $a > 13.3$ cm then

Modulus of rupture $f_b = \frac{P(x)}{b(x)} \frac{d^2}{dx^2}$ *P x l*

$$
\mathrm{If}\ a<13.3
$$

$$
f_b = \frac{3 P x a}{b x d^2}
$$

If $a < 11$, Discard the specimen

Where,

 $P =$ Maximum load applied to the specimen in kN.

L= Supported Length in mm.

$$
d =
$$
Depth of the specimen mm, and

a = Distance of the crack from the nearest support.

Fig. 4 Graphical Representation of Flexural Strength of Prism at 28 Days.

IV. Conclusion

4.1 General

In this paper the effect of silica fume on compressive strength on high strength concrete was studied by carrying out. The silica fume was replaced by 0%, 5%, 10% and 12.5% for water-binder ratio of 0.26. And also for the constant replacement of fly ash by 10% along with the above mentioned replacement. The following conclusions were obtained,

4.2 Strength Studies

- \triangleright The results of the present investigation indicate that other mix design parameters remaining constant, silica fume incorporation in concrete results in significant improvements in compressive strengths.
- \triangleright The super plasticizer demand of concrete containing Fly Ash and Silica Fume increases with increasing amount of fly ash and silica fume. The increase is primarily due to the fineness of the fly ash and silica fume.
- \triangleright The optimum 28-day compressive strength has been obtained in the range of 10% silica fume replacement level and adoption of constant 0.3% glass fibres (MSG3).
- \triangleright The compressive Strength of mix MSG3 is 1.13 times higher than that of the conventional mix.
- \triangleright The optimum 28-day compressive strength for another mix is obtained in the range of 10% silica fume, 10% fly ash and 0.3% glass fibres (MSFG3).
- \triangleright The compressive Strength of mix MSFG3 is 1.17 times higher than that of the conventional mix.
- \triangleright The optimum 28-day split tensile strength has been obtained in the range of 10% silica fume replacement level and adoption of constant 0.3% glass fibres (MSG3).
- \triangleright The split tensile strength of mix MSG3 is 1.05 times higher than that of the conventional mix.
- \triangleright The optimum 28-day split tensile strength for another mix is obtained in the range of 10% silica fume, 10% fly ash and 0.3% glass fibres (MSFG3).
- \triangleright The split tensile strength of mix MSFG3 is 1.17 times higher than that of the conventional mix.
- \triangleright The optimum 28-day flexural strength has been obtained in the range of 10% silica fume replacement level and adoption of constant 0.3% glass fibres (MSG3).
- The flexural strength of mix MSG3 is 1.18 times higher than that of the conventional mix.
- \triangleright The optimum 28-day flexural strength for another mix is obtained in the range of 10% silica fume, 10% fly ash and 0.3% glass fibres (MSFG3).
- The flexural strength of mix MSFG3 is 1.21 times higher than that of the conventional mix.
- \triangleright As the age of concrete increases, the compressive strength also increases. Silica Fume concrete attains high strength and also the concrete containing silica fume with fly ash.
- \triangleright It is proved that the high strength concrete can be obtained by lowering the water-cement ratio and the workability can be maintained by the use of super plasticizers even for very low water-cement ratio.

REFERENCES

- [1] ACI 211.4R-08, 'Guide for selecting proportions for high strength concrete using Portland cement and other cementitious materials', *Reported by ACI Committee 211*.
- [2] ACI 211.4R-93, 'Guide for selecting proportions for high strength concrete using Portland cement and fly ash', Reported by ACI Committee 211.
- [3] ASTM C-494, 'Standard specification for chemical admixtures for concrete', *Annual Book of ASTM Standards,* Philadelphia, USA.
- [4] ASTM C 642, (1994), 'Standard test method for specific gravity, absorption and voids in hardened concrete', *Annual Book of ASTM Standards, 4.02,* pp. 310-313.
- [5] Bhikshma V, Nitturkar K, and Venkatesham Y. (2009) 'Investigations On Mechanical Properties Of High Strength Silica Fume Concrete', *Asian Journal Of Civil Engineering* (Building And Housing), vol. 10 No. 3, pp. 335-346.
- [6] BIS 1959 IS 516-1959 (reaffirmed 1997) *'Methods of Tests for Strength of Concrete*, Bureau of Indian Standards', New Delhi.
- [7] BIS 1970 IS 383-1970 (reaffirmed 1997) *'Specification for Coarse and Fine Aggregates from Natural Source for Concrete'*, New Delhi.
- [8] BIS 1987 IS 12269-1987 (reaffirmed 1999) *'Specification for 53 grade Ordinary Portland Cement'*, New Delhi.
- [9] BIS 2000 IS 456-2000 (reaffirmed 2005) *'Plain and Reinforced Concrete – Code of Practice'*, Fourth Revision, pp.14.
- [10] BS: 5075 (part 3), (1985), *'Concrete admixtures – Specification for superplasticizing admixtures'*, British Standards Institution, London.
- [11] Cengiz Duran Atis. (2005) 'Strength properties of high-volume fly ash roller compacted and workable concrete, and influence of curing condition' *Cement and Concrete Research,* Vol. 35, pp. 1112– 1121.
- [12] Chandramouli K, Srinivasa Rao P, Pannirselvam N, Seshadri Sekhar T, and Sravana P. (2010) 'Study on Strength and Durability Characteristics of Glass Fibre Concrete', *International Journal of Mechanics and Solids*, vol. 5, No. 1, pp. 15-26.
- [13] Dakshina Murthy and Sudheer Reddy (2010), 'Moment-Curvature Characteristics of ordinary grade Fly Ash Concrete beams', *International Journal of Civil and Structural Engineering,* Volume 1, No 3, pp 497-508.

Ibrahim M. Metwally (2008), 'Flexural Behaviour of
- [14] Ibrahim M. Metwally (2008), 'Flexural Behaviour of High-Strength Concrete Beams Reinforced with Conventional Steel Bars and High-Performance Carbon Fibres', *The 3rd ACF International Conference*, pp.478-490.
- [15] Katkhuda H, Hanayneh B, and Shatarat N. (2009) 'Influence of Silica Fume on High Strength Lightweight Concrete', *World Academy of Science, Engineering and Technology* vol.58, pp. 781-788.
- [16] Neville A.M., (2002) '*Properties of Concrete'*, Fourth and Final Edition, Pearson Education Limited, Essex.
- [17] Paramasivam Suresh Kumar, Abdul Mannan and KurianVelluruzhathil John (2008), 'High Performance Reinforced Concrete Beams made with Sandstone Reactive Aggregates',*The Open Civil Engineering Journal*,volume 2, pp.41-50.
- [18] Santanu Bhanja and Bratish Sengupta (2005) '*Investigations On The Tensile Strength Of High Performance Concrete Incorporating Silica Fume'*, 18th International Conference on Structural Mechanics in Reactor Technology (SMiRT 18), pp.2222-2226.
- [19] Shetty M.S. (2005) '*Concrete Technology*: *Theory and Practice'*, Revised Edition, S.Chand and Company Ltd.